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ACQUISITION AND USE OF TRACK GEOMETRY DATA IN MAINTENANCE-OF-WAY PLANNING

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U.S. DEPARTMENT OF TRANSPORTATION

FEDERAL RAILROAD ADMINISTRATION OFFICE OF RESEARCH, DEVELOPMENT AND DEMONSTRATIONS The contents of this report reflect the comments and views of the B&LE Railroad, the D&RGW Railroad, and ENSCO, Inc., which are responsible for the facts and the accuracy of the data presented herein.

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Bessemer and Lake Erie
Railroad

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Denver and Rio Grande
Western Railroad

Frontispiece--FRA Measurement
Car Operations

1.0 INTRODUCTION AND OVERVIEW

Since 1971, the Federal Railroad Administration (FRA), the Denver and Rio Grande Western Railroad (D&RGW), and the Bessemer and Lake Erie Railroad (B&LE) have engaged in ^aprogram involving a joint study of the application of track geometry data to maintenance-ofway planning. The objectives of this program are to develop methods that can produce quantitative track rating information, to develop measures of track quality changes, and to utilize these techniques to establish a mathematical basis for long-range track maintenance planning.

Anticipated benefits of such ^aprogram for the railroad industry and the government include:

- Improved information for long-range maintenance planning.
- Improved control of certain maintenance-of-way budget allocations (cost to maintain track at ^agiven set of standards can be computed better).
- Establishment of quality control measures of track maintenance (which maintenance equipment or process produces a higher or a more permanent improvement in track quality).
- Improved track safety through the application of automated track geometry inspection.
- Development of computer data processing programs to process data collected by track geometry cars and to display the results in user-oriented formats tailored for different management levels.
- Provide the government with an opportunity for testing developmental track geometry measurement equipment in the railroad environment and for receiving railroad personnel evaluation of the data produced by this equipment.

In support of the above objectives, track geometry data collection runs have been made since 1971 on an annual basis (fall) on the D&RGW and on a semiannual basis (spring and fall) on the B&LE. The railroads have utilized the data collected during these test runs to make near-term track repairs and to plan certain longrange maintenance activities. In addition, D&RGW and B&LE personnel have provided invaluable recommendations for the design of new track geometry measurement instrumentation, the development and modification of data processing programs, the generation of user oriented output formats, and the establishment of measures of track quality in support of the FRA track geometry measurement car program.

The key to this program is the track geometry measurement car which FRA supplies as part of its contribution to the effort. The FRA, through its instrumentation and data collection contractor, ENSCO, Inc., supplies the measurement car personnel, computer programming capability to process the geometry data, computerized data reduction and report generation capabilities, and development of analytic techniques to evaluate track quality and the effects of various maintenance practices. The D&RGW and B&LE furnishes the locomotive power to move the measurement cars over their property, train crews, car maintenance facilities, track maintenance records, and operational and maintenance-of-way expertise for application and evaluation of track geometry data necessary for achieving the objectives of the program.

It is estimated that periodically collected track geometry covering ^aminimum of 3 years is needed to complete the development of rating indices for the various parameters of track geometry and to gain practical experience with the use of these techniques in long-range maintenance planning. This includes development of specific computer programs and output formats, collection of historical data, application of the processed output in the railroad environment, evaluation of track measurement accuracy, and modification of existing and development of new instrumentation systems.

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This development cycle is well underway for certain track geometry parameters (such as profile), but has not been started for other parameters (such as alignment).

The introduction of automated track geometry exception data and rating values provides much additional information for the maintenance-of-way officer. However, none of these data are designed to eliminate the track man or supervisor. The reports merely furnish more and better information to assist the track maintenance personnel in making better judgments.

The railroads involved in this project are both using the FRA track car measurement data for immediate identification and correction of geometry exceptions, and each has also initiated selected maintenance planning applications. For example, the B&LE, because of the length of its line and working season and the time of year of the survey, is able to use certain data reports to plan shortrange maintenance before the winter freeze. The DGRGW is surveyed once ^ayear in the fall and uses the reports mainly to review the quality of its maintenance program, to monitor the annual degradation of unworked track, and to help plan the next year's maintenanceof-way program. As a result, the geometry cars already produce data that is useful for a variety of planning purposes, in addition to exception identification.

There are several types of track geometry measurement cars operating in North America. In general, they measure the same basic parameters of track, although they do not necessarily use the same methods. This report is based on experience with the FRA Track Geometry Measurement Cars developed for the Northeast Corridor.

Operation of the FRA measurement cars on the DGRGW is the same as that on the B&LE, except that a business car is added on the end of the train. A 6-channel strip chart recorder is mounted in the rear of the business car and the roadmaster or division engineer acts as a rear observer. On the B&LE, the 6-channel recorder is

mounted in the rear vestibule of the measurement car. Any measured defects in track geometry or rough spots felt in the car are marked on the strip chart which the roadmaster may take with him at the end of his territory.

When the length of time between detection and repair is extended, as in planning large-scale maintenance programs, considerably more data must be analyzed. If the rate of track degradation is to be studied and comparisons of maintenance methods are to be made, all of the data must be examined and compared to previous surveys. This can be as many as 100 million individual data samples for one measurement run on the D&RGW. Electronic data processing and statistical analysis are indispensable tools, and the application of these tools to track geometry data analysis has been the major goal of this project. Without clear, concise, and accurate methods of presenting this information, the planner is inundated with computer printouts. Several computer programs have been written under this effort to reduce the data to manageable size. All are useful, but none are perfect.

Chapter 2 of this report describes the work that has been done in the project, the current uses of the data, and the future thrust of the study. The remaining chapters of this report are included as backup for Chapter 2. Chapter 3 summarizes and explains the different types of information reports which can be generated for maintenance-of-way personnel. Samples of the output formats are also included.

Chapter 4 summarizes an extensive testing program that was conducted to validate the FRA track geometry measurement system. The tests were performed to establish the accuracy and repeatability of measurements made with the electronic measurement and digital computer data collection systems onboard the cars. The results are discussed along with a study designed to examine the relationship of track geometry generated ride quality indices to the riding quality of track.

Finally, Chapter 5 provides a brief description of the track geometry measurement system installed onboard the FRA measurement cars. While this particular measurement system was used for this research effort, it is important to realize that other types of equipment could also be used.

2.0

DEVELOPMENT OF TRACK MAINTENANCE PLANNING CAPABILITIES

2.1 CHRONOLOGY

In the late 1960's, the FRA developed and built track geometry measurement cars to monitor Northeast Corridor tracks. The cars were designed to measure track gage, profile, and crosslevel using noncontact capacitive sensors at speeds up to 150 mph. Initial operational experience indicated that valuable track exception information could be provided to cooperating railroads through the analysis of strip chart recordings and computer printouts.

In early 1971, the D&RGW, the B&LE, and the FRA entered into a long-term research effort to develop ways of utilizing track geometry data for maintenance-of-way planning, track degradation stidies, and track maintenance quality control evaluation.

During the early track geometry development period, the strip chart recording was considered an important analysis tool, because it provided an integrated picture of the major track geometry measures. Figure 2-1 shows a portion of an 8-channel strip chart produced in real time on the FRA measurement cars. It shows nearly threefourths of a mile of track. From the top, the profile of each rail, the short chord alignment of each rail, the crosslevel of the track, the track curvature, the track gage, and track location references are displayed.

Figure 2-2 shows a location trace. Midtrack anomalies, such as road crossings, turnouts, location monuments, and even high ballast or weeds, are automatically sensed and accurately entered in the data stream by a detector located under the measurement car. Selected turnouts and road crossings have been established as reference

Figure 2-1. Typical 8-Channel Strip Chart Recording

locations and have been programmed into the computer as reference stations. The B&LE is thus divided into nearly 100 track segments of from 2 to 8 miles in length for comparison purposes. For example, the end of double track at Carter is one of these and is coded as location 19; Pine Street crossing in Grove City is coded as location 20. Pine Street is 3.91 miles north of Carter. The D&RGW has been similarly divided into track segments; but, because of the length of its lines, many more locations are required. Some of these are as much as 10 miles long where general track conditions permit homogeneous sections. By processing the data between adjacent locations, the same piece of track can be examined for comparison of one measurement run with another.

In an effort to refine the data reporting techniques and to get away from reading squiggly lines on roll after roll of paper, an off-line gage exception program was developed. Figure 2-3 is an example. The initial program output was cumbersome since, as can be seen, it noted the beginning point of a defect, each change in the severity threshold (usually every 1/4 inch), the maximum value recorded, each descending change in threshold, and the end point. Often this could mean six to 12 lines for one defect, with the report running to several hundred pages.

Figure 2-2. Example of Event Location Trace From 8-channel Strip Chart Recording

Figure 2-3. Original Off-line Gage Report

The data contained in the strip chart can be partially read and understood. For example, a short, sharp dip in the profile trace is easily seen as a low rail joint. When the gage trace exceeds 57.25 inches for the track or 57.50 inches for curved track, the roadmaster can visualize what he will find in the field. Changes in the crosslevel or curvature traces may also have some meaning to the experienced eye, however. Except for the absolute value of a low joint in profile or a wide spot in gage, the interpretation of individual traces is a function of judgment and experience. Such defects as warp, even though requiring the reading of only one trace, become more difficult and time consuming to detect. Locating areas of rock and roll in profile or areas of significant mismatch in curvature and superelevation again is a difficult process. At the same time, a meaningful comparison of one strip chart trace with another is impractical.

Computerized profile data exception reports were also under development at this time. Both profile and gage exception reports were designed to ignore good track and print out exceptions in digital form by location and severity. Figure 2-4 shows an early form of such a profile report. On the left are four columns which locate the defect. The first column designates the track being measured. The second column shows the code number of the last referenced location. The third and fourth columns show the distance in miles and feet from that location. Note that this figure is a one-page computer printout that covers 2-1/2 miles of track profile.

Columns five through eight list the defects by rail as referenced to the cars, the overall length (or distance) of the defect from cutoff threshold to cutoff threshold, and a "fact" column. An X in this last column indicates that the defect is related to a road crossing or turnout.

The cutoff threshold in this report was 3/8 inch low. More severe defects were flagged by stars, with one star indicating 1/2 inch low, two stars indicating 5/8 inch low, and so forth. The computer

Figure 2-4. Original Off-line Profile Report

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program was written to give the user the option of varying these thresholds. The right-hand half of Figure 2-4 gives various profile rating values. These were the earliest attempts at rating track quality. This program output information was the first designed for different levels of management. The supervisor or roadmaster, who is primarily concerned with defects, used the left portion, while the planner or middle management maintenance-of-way officer was concerned with the right half. From a user's standpoint, it was realized that separate reports were needed. As a result, consolidated defect summaries were developed to meet the track supervisor's needs.

The computer was programmed to sort the profile defects in a given territory by severity and print them in a sorted table. Such a listing (Figure 2-5) can include all spots above a given severity level, or can display the worst 50, 100, or even 200 exception locations. Such a printout can give a first-line maintenance supervisor all the information he needs to locate and repair the more severe profile defects. The listing eliminates the need to scan numerous pages of a more detailed report.

Profile rating summaries (Figure 2-6) were also developed for quarter-mile sections of track. These were oriented toward middle management and planning personnel. The four columns on the left of the figure define the quarter-mile track section by the location of its end point in miles and feet, either from a milepost or from a permanent location. The quarter-mile section was selected for several reasons. While one can argue that tangents and curves should be separated, neither a measurement car nor an experienced surveyor can accurately locate a point of curve to the nearest foot on the track. Some sort of permanent monument would be required at each end of each curve as an absolute minimum. The FRA measurement car can measure a mile of track with a maximum error of 5 feet (0.1 percent). The computer can accurately divide this mile into quarters. By selecting turnouts and road crossings located generally

Figure 2-5. Priority Profile Defect List (Chord Offset)

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³to 5 miles apart as permanent monuments, highly repeatable quarter-mile sections can be output for each test run. At the same time, these sections are then short enough that limited changes in the data do not become masked by the sheer mass of data.

The last column on the left quarter of the page shows the percentage of data accounted for in the section. Each quarter mile of track should have 1092 samples of profile data. If some of these samples have been lost or are not included for some reason, the rating values for that section lose their validity and cannot be used for comparative purposes unless there is compensation for this loss of data.

The first profile rating value shown is "track index." This is defined as the area between a measured profile curve and a theoretically perfect track. The numerical value for each sample of profile data is multiplied by the 2.4-foot sampling interval (converted to inches), and these products are then summed for each rail. Although individual defects are related to a particular rail, track profile quality is related to both rails. In presen^t practice, therefore, the index values for the two rails are average^d to determine track index.

As in defect reporting, the computer can be programmed to sort the quarter-mile sections by index value for longer stretches of track. These are then printed out in order of severity so that the worst track can receive immediate attention. Figure 2-7, for example, covers nearly 50 miles of track.

^Asecond method being studied to rate track is the "top of rail slope" changes per quarter mile. These are shown as "Track Slopes" in the column to the right of "Track Index" in Figure 2-6. If two consecutive data point values on either rail differ by more than 0.1 inch, the slope of the top of the rail is considered to have changed, and one slope, or slope change, is counted. The number of such changes per quarter mile is printed as a measure of surface quality.

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Figure 2-7. Priority Profile Defect List (Track
Segment Summary-Index)

^Athird profile rating technique is printed near the center of Figure 2-6 and is denoted as "Cumulative Slope." To define this concept graphically, the cumulative histogram (Figure 2-8) of all the profile data points in a quarter-mile section of track is constructed by the computer. The X-axis shows the profile values, while the Y-axis shows the sum of all the data readings equal to or less than that profile reading as a percentage of the total samples.

Mathematically the slope of a line is described as the tangent function which is equal to the difference in Y-values divided by the corresponding difference in X-values. By reading across the constructed graph at the 70 percentile level to the curve and then down to the X-axis, the corresponding profile value can be determined. The same can be done at the 30 percentile level. The change in Y-values is 70 percent minus 30 percent, or 40 percent. The change in X-values is the algebraic difference of the two Xaxis readings. The slope value can therefore be calculated by division.

If the track section is smooth, the data points will tend to group tightly together; the change in X-value will tend to be smaller, and the slope value will tend to be larger. If the track section is rough, the data points will spread more across the graph; the change in X-value will be larger, and the slope will tend to be smaller.

These rating methods are concerned with the overall condition of ^aquarter-mile section. However, it is possible that only a few isolated spots may begin to deteriorate. These could be at road crossings, at insulated joints, or at turnouts. It is also desirable to flag this type of change. The 0.5 percent value shown in Figure 2-9 is an attempt to note this type of change. This can be described as that value of the profile data which is less than 99.5 percent of all the values in the section. The figure shows the lower end of the cumulative distribution curve,

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Figure 2-8. Cumulative Slope Value Calculation

with the cumulative percentage of the data as the ordinate and the profile value as the abscissa. The computer, in effect, reads up to the 0.5 percentile level and then over to the curve to find an intercept value. This is the 0.5 percent value and is printed **out** in the summary data.

The calculation of the 0.5 percent value normally utilizes five data samples, or 12 feet of track. If these samples represented ^a "critical" defect, they might be corrected before the computerized data analysis was complete. For future comparative purposes **it** would, as a result of corrective actions, lose its meaning.

PROFILE DATA -.5% VALUE

PROFILE IN **INCHES**

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Pictorial representations of the data can be made by the computer through the use of the histogram. In Figure 2-10, the percentage of profile data points in each cell, shown on the Y-axis, 1s plotted against the profile value for the cell, shown on the X-axis. The histogram can be an effective technique for displaying data for longer stretches of track.

As with the study of track profile, it became necessary to develop gage rating techniques and to output them in a separate useroriented report. Figure 2-11 shows gage rating techniques used to evaluate approximately four miles of track.

Figure 2-10. Typical Profile Histogram

Figure 2-11. Gage Track Segment Summary

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The track has been divided into the standard quarter-mile sections during data processing. The columns on the left of the figure locate the end of the section and identify the particular track. Perhaps there is too much data displayed here merely to locate the section, but to a large extent this must be a decision of the user.

In the next column the "mean" gage is given. This is the arithmetic average of all the data points recorded in that quarter mile of track. Normally this is 546 readings. A glance at the "mean" gage readings on this figure will show that it varies from 56.24 inches to 56.81 inches. An average gage of 56.24 inches may be questioned by some track maintenance men, but this is continuous welded rail which was laid 1/4-inch tight on tangents. A "mean" gage of 56.81 inches is, however, questionable.

^Asecond gage rating value which was developed is the gage "variance." This is the normal statistical "variance," or the square of the standard deviation, of all the data points within a quarter-mile section. Any offset of the data due to calibration is ignored by this calculation. The "variance" is shown near the center of the figure for each quarter mile of track.

Both "mean" gage and gage "variance" can be used to examine the general condition of the gage in a quarter mile of track. However, ^alimited number of spots where gage may be changing are also cause for concern. Just to the right of the "variance" data are six columns labeled percentage levels. These indicate the percentage of all the data samples in that quarter-mile section which fall within the gage value groupings shown at the top of the page. ^A number of the track sections shown have 100 percent of their values located in the first percentage level, or below 57.25 inches. Most sections fall in the first and second levels, or below 57.50 inches. All of the sections fall within the first three percentage levels, or below 57.75 inches. This is good Class 4 track.

The percentage level rating technique actually looks at both the broad trends in overall gage and the limited changes at the wide end of the distribution. As such, it could be a valuable technique.

At the right of the figure are three columns which are headed "percentage values." These are developed in the same fashion as the 0.5 percent values for profile. The computer is looking at the wide gage side of the distribution. Three different levels have been computed and printed out in an effort to determine if one is more significant than another.

In the historical development of the rail research program, the next step was the generation of a crosslevel exception program. Although a crosslevel system was operational on the measurement cars from the beginning, more than a year passed before attempts were made to write a program locating and quantifying the defects. Because an isolated crosslevel value may or may not be a defect, depending on whether the track is tangent or curve, it was necessary to provide the computer with the logic equivalent to the judgment and experience that the track maintenance personnel would use when looking at crosslevel data on a strip chart.

This was accomplished through the use of a data sample averaging technique. The values of the 60 data points immediately preceding the data point in question as well as the 20 data points immediately following are averaged. If the value of the data point in question varies more than a preset threshold amount from this average, it is considered a defect. As such it can then be output in a digital crosslevel exception report. It can be displayed as a point or as a longer exception having a beginning, a maximum, and an end The defect is also specified by its location in the track.

Figure 2-12 is an example of one of the earliest of such reports. At this time, curvature output information was attempted using the crosslevel data. This information is shown in the left center of the figure. After a small amount of field analysis, it became

Figure 2-12. Original Off-line Crosslevel Report

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obvious that this portion of the report was too inaccurate to have ^apractical use in curve analysis or maintenance. A curvature system and a report based upon its data would be needed.

On the right third of the figure crosslevel and warp exceptions are listed. While most of the crosslevel exceptions could be verified in the field, they represent only those spots where the actual data value varies from the average. Crosslevel in tangent, reverse elevation, and improper elevation of curves were beyond the measurement capability of the system. This situation provided the motivation for developing an independent curvature measurement system and curve evaluation report.

About this time an extremely valuable tool was made operational on the measurement cars (Figure 2-13). This was the real-time digital exception printout. It furnished the maintenance-of-way supervisor with a hard copy list of "critical" defects by magnitude and location which he could take with him when he left the cars. A second copy was provided for office use or as ^achecklist of corrected defects reported from the field.

This list has been successfully used by track maintenance forces. There have been several instances when a track supervisor, working from such a list, corrected the "critical" defects before a formal list could be given to him from off-line data processing runs. On one occasion, when the on-line digital tape recorder malfunctioned, gage and profile corrections were made by working from the real-time digital exception printouts.

One objective during the course of this study was the simplification of reports and a reduction in the amount of generated report paper. Figure 2-14 is an example of the first attempt to develop an offline integrated exception report showing the defects in all parameters on one page. This report eliminated the need for separate gage and profile exception reports, allowed the track man to see how defects related to each other, and did away with the cumbersome switching from data book to data book.

Figure 2-13. Typical On-line Exception Listing Report

The earliest form of the integrated exception report included profile, gage, crosslevel, and warp data. It was designed as an "ouch" type of report for first-line track supervision. Some of the holdover information from earlier reports can be seen. Not only is the defect magnitude and location printed out, but the beginning and often the end are also located. Such a technique can double or triple the amount of paper without conveying any additional information.

Figure 2-14. Original Integrated Exception Report

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The development of a curvature measurement system led to the generation of a curvature defect report. As with crosslevel, there is no way to tell that a single, isolated curvature reading constitutes a defect. Therefore, the data sample averaging technique that was developed for crosslevel was adapted for curvature defects. If the absolute value of the curvature data sample varies more than ^apredetermined amount from the average, it constitutes a defect. This technique can be used equally well in tangent, curve, or spiral track.

Field testing of the curvature system has shown it to be accurate. Figure 2-15 shows the string line measurements for two curves ^plotted to the same scale as the strip chart traces. For practical purposes, the string line measurements and chart traces are identical.

Figure 2-15. Comparison of Curvature Data from String Line and Curvature Subsystem Measurements

The same curve was measured by the curvature system at varying train speeds (Figure 2-16). In this figure the speed was varied from 20 mph to 50 mph in 10-mph increments. The resulting strip chart traces are identical.

By combining the inputs of the curvature and crosslevel systems into one program, the curvature analysis report shown in Figure 2-17 was developed. This is not strictly a defect type report. The left third of the figure locates and describes the curve without regard for defects. Neither does this portion of the repor^t specifically attempt to rate the curve. It locates the event, identifies the point of tangent to spiral, the length of the receiving spiral, the point of spiral to curve, the length of curve, the average degree of curve, the average superelevation in the

Figure 2-16. Comparison of Curvature Subsystem Data Collected at Various Speeds

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Figure 2-17. Curvature Analysis Report

curve, the limiting speed in the curve due to superelevation defects based on FRA track standards, the allowable speed in the curve based on average superelevation as determined from FRA standards, the point of curve to spiral, the length of the leaving spiral, and the point of spiral to tangent. This is a tremendous amount of information. The need for some of it may be questioned, but the data must be developed in order to determine curvature and superelevation defects.

The right-hand two-thirds of Figure 2-17 is strictly a defect report. Errors in matching crosslevel and curvature are noted. These can occur at the tangent end of the receiving or leaving spiral when curvature and superelevation do not begin or end at

the same point. This mismatch can also occur at either end of the body of the curve when full curvature and full elevation do not coincide. If a mismatch of more than a preset distance exists, the length of the mismatch is located and printed out as well as the maximum error in crosslevel. In Figure 2-17, the mismatch distance threshold is 50 feet.

Reverse crosslevel can be found in the tangent just ahead of the spiral, when the crosslevel actually goes negative (with respec^t to the superelevation of the curve). If this condition is sensed above the preset defect threshold, a defect is printed showing its magnitude and location.

^Acurvature defect is sensed by comparing the actual data point value with the running data sample average. In Figure 2-17, the defect threshold is 1-1/2 degrees greater or less than this average. Again, the magnitude and location of the defect are shown. The maximum allowable speed for the defect is also computed from the formula in the track safety standards and shown next to the curvature defect.

Superelevation defects in curves are shown. These are computed as previously discussed in the determination of crosslevel defects.

Warp can be defined as the rate of change of crosslevel. In Figure 2-17, warp is computed on ^a31-foot length, using a 1-1/4 inch defect level for tangents and curves, and 1 inch for spirals. These values can be easily changed in the program to meet the needs of the user.

The periodic irregularities in crosslevel which can produce rock and roll instability can now be computed. In Figure 2-17, it is analyzed over ^a19-1/2 foot distance. Given a particular data sample, the program examines the data sample a preset distance away. If the difference in crosslevel between the two samples is above a selectable threshold, the sign of the slope between the

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two points is saved. Using the preset distance, a new crosslevel difference is examined and compared to the threshold. If it exceeds the threshold, a second slope is calculated; if the sign of the second slope is opposite to that of the first slope, two changes in direction have occurred. If a continuation of this analysis produces a third slope, having a sign opposite to the second slope, a possible rock and roll situation is considered to exist and it is printed out and located. The critical thresholds used for rock and roll in this example are equivalent to three or more successive spots which are 5/8 inch low on alternate rails and 19.5 feet apart. Again, the thresholds can be varied for the user.

In both curvature and crosslevel reporting, a sign convention was developed to identify the direction of curve of the high rail. ^A positive sign indicates that the curve is to the east, or that the west rail is high. A negative sign indicates that the curve is to the west, or that the east rail is high.

The exception report currently being produced for and used by the railroads to display geometry defects is an improved version of· the Integrated Exception Report (Figure 2-18). This is a comprehensive report that displays, by location, profile, curvature, gage, crosslevel (or superelevation), warp, and rock and roll defects on one page. Normally several miles of track geometry defects can be displayed on one sheet. Curvature has replaced the alignment column which was listed and left blank in the ori^ginal integrated report. To date, it has not been possible to obtain consistent rail alignment data from the 14.5-foot capacitive sensor chord and to interpret it properly. Only one warp is shown, and this is based upon a user-selected warp distance. In Figure 2-18, the selected distance is 31 feet. The second warp value originally displayed in the integrated exception report has been replaced by rock and roll. This, too, is based upon a userselected frequency distance and is set at 19.5 feet in the example. ^Afact column has been added to the extreme right of the printout. This informs the user that the defect in question occurs in a tangent or in a curve, in a road crossing, or at a turnout.

Figure 2-18. Current Detailed Integrated Exception Report

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One mile summaries have been added to locate track sections by mileposts and to show the number of defects in each parameter within that mile. Two exception levels are available and are user selectable. One might be termed a normal defect level, while the second might be considered ^asevere defect level. Severe defects shown in the summary listings are identified by stars in the detailed section of the report. An example of this summary portion of the report is shown in Figure 2-19.

Most of the defects displayed in the Integrated Exception Report are not based upon FRA track safety standards. For many parameters they can be if the user so desires. The FRA track safety standards define minimum requirements for safety. The railroad industry should theoretically be able to maintain track at some higher standard. However, it is not practical to set defect levels as deviations from perfect track. While "critical" (within the context used here) defects cannot be ignored and must be identified and corrected for the safety of the trains, the maintenance-of-way officer has a broader responsibility to his company. He must determine that the dollars allocated for maintenance are spent where they are most needed and that the maximum benefit is obtained from the work done. The cost-benefit ratio required to maintain theoretically perfect track would be prohibitive.

In practice, there are probably three general levels of management to which track geometry data reports should be directed. The first level is the track supervisor or roadmaster, who is primarily concerned with defects. Middle management (the division engineer and ^planning engineer) are not overly concerned with individual defects. Instead, they are more involved in the general quality of the track and the changes taking place in that quality. The geometry rating reports are directed to them. Upper management needs a report that can give a clear and concise picture of the condition of the track and changes in that condition.

Figure 2-19. Current One Mile Summary of Integrated Exception Report

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PROCESSED ACQUISITION 10/13/74

ONE MILE SUMMARY EXCEPTION REPORT

PAGE 1

TRACK NUMBER # 2

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GEOPLOT was developed for use by the latter two groups. The example shown here (Figure 2-20) gives a 2-year (or a two successive measurement run) quality rating comparison of gage, profile, crosslevel, and curvature. The ratings are displayed by miles, with approximately 50 miles of track being displayed on one page of computer printout. A maximum of three runs can be presented on this plot.

In order to output such a program, it was necessary to have quality rating values for each of the four parameters. Variance, or standard deviation, was already available as a rating of gage. Track index had been developed as a rating of profile. But no rating systems existed for crosslevel and curvature. These were needed both for GEOPLOT and for investigations of quality changes.

By applying the same general concept that is used to compute the profile index, crosslevel and curvature index values were programmed. In profile index the actual data sample value is compared to theoretically perfect track, and the area under the difference curve is computed. In crosslevel and curvature index, the actual data sample value is compared to an average value. Differences between data sample and averages, when multiplied by the sampling interval and summed over a mile of track, give the index or rating value.

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COMPARE is the most recent program to be developed. It is an attempt to give a pictorial record of changes in track through the use of the comparative histogram. In the example (Figure $2-21$), one set of data is represented by a "1", while a second set of later data is noted with a "2". Common points are denoted as dots. Track segments of various lengths may be examined, with changes being easily discernible.

2.2 APPLICATIONS

From its inception, the railroads have considered the track geometry study to be a research and development project. Even though the measurement cars had operated on the Northeast Corridor for several years and had been used elsewhere on occasion, they were new to the B&LE and the D&RGW. The results of the first measurement runs over

Figure 2-20. GEOPLOT Report

TRACK PARAMETER QUALITY INDEX CHART

 $GAGE$

 S_{TO} S_{TO} DEV_{\bullet}
INCREMENT = .02

MP603 TO MP741 $3 = 1973$ DATA $4 = 1974$ DATA

 $(1 + 5)$
 $(1 + 1)$

 $(LOC/M_{\bullet}P_{\bullet})^{-\phi}$

 $\ddot{}$

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 $\ddot{}$

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PROCESS DATE 12/19/74 X=2 OR MORE POINTS THE SAME
N=8AD OR NO DATA

 $\frac{1}{2}$ $\frac{-34}{2}$ x

CROSSLEVEL

INDEX
INCREMENT = 650.00

PROFILE

 $INCREMENT =$ 50.00

 $\frac{---x}{---34}$

PAGE 1

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CURVATURE

 $INCREMENT = \frac{INDEX}{1150*00}$

 $\frac{-1}{2}$ - 3-4

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Figure $2-21$. Typical Histogram Comparison Report

tracks of these railroads were presented as strip charts and as profile and gage defect reports. These strip charts were taken into the field, and many of the more severe defects were located and verified before the track maintenance forces were given the data to use in making repairs. As a result, these people have accepted the system and its outputs with few reservations. Today they will ask when the next measurement run is scheduled, or what the last run might have indicated concerning a particular piece of track. They are convinced of the basic validity of the track geometry measurement system, and this proved helpful in developing acceptance of the project.

One of the original requirements of the FRA geometry measurement cars was that they have the ability to test at speeds up to 150 mph. This stipulation necessitated the elimination of the contact sensor for measuring gage. As a result, the capacitive sensor was developed. Although these sensors do not make contact with the rail, their

proximity to the rail head makes them susceptible to damage, particularly at road crossings, self-guarded frogs, and raised guar^d rails. Because they function on an electrical capacitive principle, water on the rail or on the sensor will tend to give faulty readings. The capacitive sensor, then, is definitely a fair weather type of instrument. All-weather sensors are presently under study by the FRA and will be field tested during 1975.

Unfortunately, difficulties with the capacitive gage measurement system have hindered the development of gage rating techniques. At first "mean" gage appeared to be a promising rating value, but after two years of experience, analysis, and field checking it was found that the measurement system could not repeat gage accurately enough from year to year to allow for comparisons.

Calibration errors, lipped or worn rail, and the nonlinearity of the voltage-gap curve in the amplifiers can give a probability of error in gage readings greater than the actual change in gage in the course of the year. This is not to say that the absolute value output for a gage defect is not valid. In reality it is. Experience over the last 3 years has shown that virtually all of the defects reported are real.

For comparative purposes, gage "variance" has proved to be a more meaningful indicator of a general change in gage. Since it is ^a measure of the variation from average gage, it is not nearly so sensitive to calibration errors. An analysis of repeatability tests made with the measurement cars has shown that a change in the gage "variance" value of 0.0013 can be interpreted as a meaningful change in the gage of that section of track. One consideration in the use of gage "variance" as a rating value is the curve, or section of track, where the gage could conceivable widen uniformly. In such a case the "variance" value might not change because the "mean" and not the scatter of data 'about the "mean" had changed.

Use of the percentage level rating technique for gage has been hampered by the same instrumentation problems that developed in the attempts to apply "mean" gage for comparative purposes. However, the calibration oriented shifts are somewhat more apparent in the percentage level technique. Continued study of this technique is planned.

Some of these problems will not be solved until the all-weather gage system is installed and field tested. But another change is being considered for the gage reports. At the present time, the reports do not consider tight gage, on the assumption that trains are safely passing over that portion of track and gage does not normally tighten. There appears to be merit in adding a column in the quarter-mile rating summary report to show a 0.5 percent value for gage data similar to the 95, 97, and 99 percent values. This would look at the extreme narrow portion of the distribution curve, and would satisfy safety requirements.

To date, the exception reports have been used to correct gage defects after each measurement run. They have also been used for several years to maintain safe gage on a light traffic branch line even though a tie deterioration problem is developing. Both the defect reports and the gage rating values have been successfully used to assist in the upgrading of another branch line for anticipated heavy tonnage increases.

It has been possible to show progressive gage widening in a few isolated curves by comparing gage rating values over a period of several years. In this process there have developed indications that the mechanized tie gang may, in some cases, actually contribute to long range gage widening. The gage on a curve may slowly widen, either uniformly or in spots, up to 1/4 or 1/2 inch between tie renewal cycles. The tie gang is apt to renew ties without noting a change in gage. If such a set of conditions continues for several cycles, the roadmaster may suddenly discover a curve which has a l-inch-wide gage although the track is well spiked on generally good ties.

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Location and correction of profile defects now require the use of the real-time exception printouts and the Integrated Exception Report. While this may appear to be a duplication of data, each
report furnishes somewhat different information. The real-time report furnishes somewhat different information. exception printout indicates "critical" defects that need immediate attention. The Integrated Exception Report lists considerably more defects (many of lesser magnitude) as well as exceptions sorted by severity. A "critical" defect in the context of this paper does not mean an impending derailment, nor does it indicate a violation of the FRA track safety standards. "Critical" defect levels were established by the chief engineer as a statement of minimum acceptable track quality for his staff to follow.

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More progress has been made in the rating of track profile than in any other parameter. Some of these rating values are in actual use today. The profile "index" value is one of these. Profile "index" values have been found to range from 600 to 2,000 square inches per quarter mile of continuous welded rail. On bolted rail on branch lines, "index" values can go as high as 3,500 square inches per quarter mile. An analysis of the repeatability tests made with the measurement cars has shown that a change of 125 or more square inches in the "index" from run to run can be interpreted as ^ameaningful change in profile quality.

The index value has also been found informative. A measurement run was made on the Hilliards Branch track (Figure 2-22) in the fall and repeated the following spring. Between runs the track was raised and lined from Milepost 5 to Hilliards. Changes in the computed index value showed an expected deterioration between Milepost 1 and Milepost 5. From Milepost 5 to Hilliards most of the track showed a definite improvement. However, between Milepost ⁶and Milepost 7 the track profile seemed to have become rougher, even though it had been raised.

	Index Per Mile				
	Spring 1973	Fall 1972	Change	% Change	
$MP H-1$ to $MP H-2$	6,511	5,514	$+997$	$+18%$	
$MPH-2$ to MP $H-3$	7.526	6,446	$+1,080$	$+17%$	
MP H-3 to MP H-4	7,820	6,426	$+1,394$	- 22%	
MP $H-4$ to MP $H-5$	7,028	6,284	$+739$	$+12%$	
MP $H-5$ to MP $H-6$	5,020	5,484	3464	$-8%$	
$MP H-6$ to MP $H-7$	6,488	5,926	4512	$-8%$	
$MP H-7$ to MP $H-8$	6,099	7,157	$-1,058$	BIS%	
$MPH-8$ to MP $H-9$	5.996	EAKEP	22,486	BDOM	
Maiero interza	0.00	BASE	BORTO	BEATA	

Figure 2-22. Example of Track Index Value Application (Hilliards Branch)

^Adiscussion with the track supervisor for this territory revealed that surfacing actually began near Milepost 6 with the gang working toward Hilliards. They then returned and raised between Milepost ⁵and Milepost 6. This was the first use of the tamper since its winter overhaul, and problems developed in its electrical system. For the first several days of surfacing, production was low and the machine had a tendency to hump, or overraise, the track. The problem was corrected by the time the gang passed Milepost 7.

Each fall the Track Department is given a list of quarter-mile track sections (Figure 2-23) with high index values, so that these sections can be smoothed before the winter freeze sets in. These sections are taken from the track "index" value summary reports which are sorted for severity.

	(Track 100) for 1/4 mile south.	TO 94
	northward 1/4 mile thru siding switch.	1678
A.	From 100 feet south of house switch at Girard for 1/4 mile north thru #9 switch.	1970
	northward for 1/4 mile.	1018
$\mathcal{L}_{\mathcal{A}}$	From south edge of Broad Street crossing at Grove City to 500 ft. north of Mill Street.	Frit
	for 1/4 mile north.	LEOD
Ч.	From 1500 feet south of Odd Fellows crossing (MP 61) for 1/4 mile south.	1765
8.	mile south.	17. G
Ω.	From 150 feet south of Odd Fellows crossing (MP 61) for 1/4 mile south.	NEO
TO.	From south end of Main Street crossing at Fredonia for 1/4 mile south.	WIC
		1. From 500 feet south of Bull Barn crossing 2. South end of Main Street crossing at Fredonia 4. 1600 feet north of north end of Kimble viaduct 6. From 400 feet north of MP 67 (south of Pardoe Thru Coolspring cripple switch and for 1/4

Figure 2-23. Example of Track Segment List Used for Maintenance Planning

The "track index" values and the "cumulative" slope values tell an almost identical story. Quarter-mile values (Figure 2-24) are plotted here for 3-1/2 miles of actual track. Index values are dark, and slope values are light. A correlation analysis of the two values has been made, and the results support the evidence ^pictured here. A further discussion of this correlation analysis can be found in Chapter 4 of this report.

Presently, these "index" values are also being used by the maintenance-of-way department to locate and verify a significant portion of its annual surfacing program. They are also being used, particularly in the spring, to determine quarter miles of track which need to be improved to the quality of the longer stretches on either side of them.

To date, profile defect and rating data have been developed by the capacitive sensor chord system. Thus it is not compatible with FRA standards. In its present state of development, the profilometer (selected as the replacement instrument for the capacitive profile system) appears to be speed sensitive and has exhibited some wide discrepancies between consecutive runs over the same track at different speeds. However, recent tests indicate an early solution is expected. Once profilometer accuracy has been demonstrated, ^plans call for developing profile data from this system. However, rating values from the two systems are not entirely compatible, thus both systems will be required for a year or more to insure that comparable data are available over a period of three or four measurement runs. Because the profilometer can output data in terms of the FRA track safety standards, a major effort is being directed toward correcting its problems and verifying its outputs.

Figure 2-24. Quarter-Mile Rating Value Comparison

Profile "index," slopes, and cumulative slope values are duplicate measures of track quality. At least one of them will probably be discontinued at some time in the future. All rating values are designed to reduce many data samples to one or two numbers. This condensation is mandatory for run-to-run comparisons and for summary reports to higher management. To some extent, this brevity may be paid for by an accompaning loss of some of the information. Each of these three values must be studied in much greater detail to determine which presents the greatest amount of usable data in the most concise form.

At present the severe defects in crosslevel, warp, rock and roll, and curvature are taken from the Integrated Exception Report and checked in the field. Corrections are then made by the track forces. Experience has shown that the reported defects are valid imperfections that need attention. Even though the raw data were taken under load, a large majority of the defects can actually be seen by eye in the field if the investigator knows where to look. However, a track inspector would normally pass over many of the smaller defects without the use of the report.

Aside from the crosslevel and curvature "index" values used to develop GEOPLOT, no rating values for these parameters have yet been devised. Crosslevel and curvature "index" values have ye^t to be verified and significant values of change determined. There has been no effort to output these values for the quarter-mile track sections for comparative purposes. This effort will be ^a future project.

The curve definition portion of the curve analysis report is difficult to validate. Not even an experienced surveyor can determine if a curve begins exactly at a given physical point. An ingenious method of validation was recently suggested. Although the length of curve and the degree of curve can be changed, the central angle for that curve does not change as the result of maintenance. It is established by the angle of intersection of the tangent on either end of the curve.

The central angles of all the curves in a rail line are known from the days when the railroad was built or from the data of the most recent line changes. If the central angles for a series of curves are computed from the data in the curvature analysis report, the amount of agreement with the known central angles of these curves is an accurate measure of the correctness of the report. Preliminary work has been started in this area.

But a number of valuable, longer range observations have already been made from curvature and crosslevel data. Instances of crosslevel in tangents, while not of sufficient magnitude to be safety standard violations, indicate that the quality of production tamper raising may not be as good as has been assumed. This may be the result of equipment weakness, human failure, or both. Swings of up to 1-1/2 degrees with a definite periodic frequency have been found in isolated curves. Here the maintenance officers would not believe that they existed until a set of stringline notes proved otherwise. This condition appears to have been the result of curve deterioration resulting from traffic.

A study of the matching of superelevation with curvature, particularly in the spirals, indicates that raising and lining techniques need some improvement. The point of full elevation often does not match the point of full curvature. The beginning point of the spiral does not always match the point of zero crosslevel, yet generally the spiral curve is smooth and the superelevation runout is uniform. It has been suggested that the lining machine, measuring curves ahead of the tamper and then struggling to keep the lining operation close behind the raising gang, cannot show the raising foreman the proper points for starting and ending superelevation. Each foreman must use his best judgment in locating the proper points.

The GEOPLOT program appears to be a very useful summary. The indices are based on statistical values which are relatively insensitive to possible instrument error. Since large-scale maintenance programs are planned for at least one mile of track, any

errors in distance measurement would not be great enough to create ^aserious comparison problem. It gives a snapshot summary of the major parameters to the maintenance planning engineer.

Because the COMPARE program is relatively new, its usage in practical applications has been limited. Its chief benefit appears to lie in its ability to picture track geometry changes in a form that many people can readily understand. In its present form some of the values, particularly gage and profile, are quite sensitive to instrument error. The COMPARE program must lump longer stretches of track together to reduce the required paper to manageable proportions. Even then its output of sheets of paper is many times that of GEOPLOT.

2.3 FUTURE CONSIDERATIONS

It has been claimed that, given the tremendous amount of data developed in one measurement run, reports exceeding the length of the track being measured can be generated. In fact, one of the primary objectives has been to condense and simplify reports. The 1971 gage defect report actually contained almost as many pages as all of the 1974 reports combined. Much has been learned about instrumentation, computer processing, and report generation. Much remains to be learned, but the project is well on its way to developing useful measures of track quality as well as measures of change caused by service and/or maintenance practices and to present this information in a concise manner.

The development of new modified track geometry instrumentation must be continued to permit the final development of effective maintenance-of-way planning techniques. The installation and verification of the all-weather gage system must be done before further progress can be made in the study of gage deterioration. Until the profilometer is modified and its output verified, profile defect data cannot be expressed in terms of the FRA track safety standards. The same situation appears to be true if rail alignment is to be

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compared to federal standards. An alignometer, which is under development, appears to hold considerable promise in this field. This device would function much like a lateral profilometer. Under study is the possibility that an alignment standard's comparison program can be written for the existing alignment data.

The programs that were discussed earlier are under continuing study and change. In this area the object is to output more usable information, reduce the amount of paper involved, reduce the cost of programming and data processing, explore the possibility of real-time processing of all data, and provide each level of management with the most useful reports.

Particularly in the area of profile, some quality control value is already apparent in the rating summaries. Curvature and crosslevel still require the development of verified rating techniques. Some of these are already developed, but there is much more study needed.

As of this time, no attempt has been made to develop any costbenefit relation using the track geometry data. This aspect of the study is probably several years in the future.

At present, there are limitations to the use of track geometry data. For example, it can never replace the track man. If track begins to deteriorate, the measurement system can tell where and in what parameters, but not why. A track man must look at that location and use his experience and judgment to determine corrective solutions. Sophisticated systems can only produce valuable information to aid in the decision making process.

Track geometry is not an end in itself. The track structure is merely the stimulus which excites a dynamic response in the railroad car. While some cars can develop undesirable dynamic reactions on poor track at low speeds, other cars can show dangerous excitation on relatively good track at higher speeds. As more is learned about the reactions of car suspensions to the stimulus of track, railroad safety will depend less on "operator judgments" and more on applied technology. The use of automated track measurement systems should then result in safer and more economical railroading.

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3.0

MAINTENANCE INFORMATION REPORTS

3.1 INTRODUCTION

Post-run processing of digital track geometry data is performed to provide track maintenance information reports in a format useful to maintenance-of-way personnel. A number of different reports have been developed to evaluate track geometry data. They include:

- Crisis Maintenance Report which highlights exceptions requiring immediate action (includes location and magnitude).
- Detailed Exception Listing which provides detailed information on the magnitude of each exception (gage, profile, crosslevel, curvature, warp, or rock and roll) keyed to geographic location.
- Maintenance Planning Report which presents information on the average quality of track sections in a manner useful for maintenance scheduling and for the evaluation of maintenance performance. This report lists track quality measures and contains summaries that rank sections of track according to quality.
- Data Comparison Report which presents information useful for evaluating the rate of track degradation, the effects of specific types of track maintenance, and the performance of track maintenance equipment by means of **(1)** histograms for two sets of data which are superimposed to give visual representation of differences in the measured segments of track, and (2) geographic plots where quality measures for selected track geometry parameters are used.

These reports are used to present detailed maintenance information on gage, profile, curvature, integrated exception (a single report summarizing gage, profile, curvature, crosslevel, warp, and rock and roll exceptions), and integrated standards (a report summarizing exceptions to federal track safety standards).

Table 3-1 displays a list of all the reports that are generated for maintenance planning. For each report the table shows which parameters are analyzed and which reports are used by each level of management.

3.2 GAGE REPORTS

The Gage Program produces a multipurpose report which is oriented toward railroad maintenance activities. The report contains data for crisis maintenance, detailed exception analysis, and maintenance planning.

The Gage Critical Threshold List (Figure 3-1) contains a summary of critical gage exceptions found during data acquisition. Critical gage thresholds can be predetermined in order to give the railroad user ^alist of exceptions which he considers critical. The list is not ordered by severity, but is merely a summary of anomalies encountered. Each entry line includes information which is used to determine the geographic location of the exception (i.e., milepost distance) as well as the distance from the reference location.

The Gage Priority Defect List, also called the Largest Exception List (Figure 3-2), is a one-page summary used for gage crisis maintenance. It lists the one hundred largest gage exceptions in order of decreasing magnitude.

The purpose of these two critical exception reports is to provide the individual railroad user with sufficient information to determine where to concentrate spot maintenance efforts.

Figure 3-3 illustrates the relationship between the strip chart recording of gage and the computer-generated gage exception listing. When a gage value exceeds a selectable low threshold, such as 57.00 inches, a gage exception is indicated. If only a single value within ^aspecified distance is in excess of this threshold, a single line entry will be printed.

TABLE $3-1$

RELATIONSHIP BETWEEN MEASURED PARAMETERS,
MAINTENANCE REPORTS, AND MANAGEMENT LEVELS

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Gage Critical Threshold List Showing
All Defects over a Severe Threshold Figure 3-1.

Gage Priority Defect List Showing the 100 Largest
Defects Sorted by Decreasing Order of Magnitude Figure 3-2.

Figure 3-3. Relationship of Detailed Gage Exception Infor-
mation and the On-Line Strip Chart Display

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If gage values within a specified distance ahead also exceed the low threshold, ^agroup exception will be printed, grouping all exception data ahead until no additional data exceeds the low threshold within the grouping distance. The final group exception entry contains all of the data printed for the single exception mentioned above, plus the word END in the Comments column.

There will always be a START and MAX entry to indicate the beginning and maximum points of an exception group. In addition, there may be entries to indicate the change from one severity level to another.

There is no distinction between wide gage and tight gage exceptions other than in grouping (all wide gage exceptions appear in one report section, tight gage exceptions in another) and in the directionality of surpassing a threshold.

The purpose of the Gage Track Segment Summary (Figure 3-4) is to display an analysis of the quality of the track in terms of the gage parameter. The method of analysis is to calculate various statistical measures for each segment (normally one-quarter mile, unless otherwise specified).

Each entry in the report represents a track segment. The distance information for mileposts and reference locations indicates the point at which the track segment terminates. For each segment, the mean and variance of the gage data are calculated and printed. Included also is a distribution chart for all of the valid data within the segment. The latter is generated by setting six categories for the gage data and determining into which one each measurement falls. When all measurements have been categorized, the number of measurements in each category is presented as a percentage of the total number of measurements made in the segment. Thus each track segment entry in the report shows data percentages for each of the six levels. Three percentage values (95, 97, 99) are extracted from the distribution chart to give an indication of gage width within a given track segment.

Figure 3-4.

Gage Track Segment Summary Showing an Analysis of
Track Quality of Gage by Quarter-mile Segments

The value of this analysis is twofold. First, it is possible to quickly determine the relative quality of one track segment compare^d to another. As percentages in higher categories increase, the quality of the gage decreases. Second, when analyzing data from one run to another, it is possible to make comparisons of track segments in terms of the respective distributions of their gage measurements. By doing this it is possible to determine whether the gage has widened appreciably in a specific area.

Gage Histograms graphically illustrate the condition of gage by showing the distribution of gage measurements. Gage data histograms are presented with measurement percentages on the ordinate axis and incremental gage values on the abscissa. The sum of the measurement percentages totals 100 percent. Although calculation of the mean gage and other statistics can be performed from the histogram, its primary purpose is to give railroad personnel a graphic comparison of sections of track. As an example, there is little doubt of the relative quality of the track segments represented in Figures 3-5 and 3-6.

The Gage Program provides individual histograms for each reference location (normally about 5 miles of track) and a summary histogram for all of the data analyzed.

3.3 PROFILE REPORTS

The Profile Program produces a multipurpose report which is oriented toward different levels of maintenance activity similar to the Gage Program.

The Profile Critical Threshold List (Figure 3-7) contains a summary of critical profile exceptions found during data acquisition. The thresholds can be predetermined in order to give the individual railroad user a list of profile exceptions which he considers critical. The list is not ordered by severity; rather, it shows each critical anomaly that is encountered.

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Figure 3-7. Profile Critical Threshold List Showing All Defects
Exceeding a Critical (Set by Individual Railroad
User) Threshold.

The Profile Priority Defect List (Figures 3-8 and 3-9), also called the Largest Exception List, contains three pages. The first page lists the one hundred largest profile chord offsets for the two rails. Pages two and three list the fifty most severe track segments based on "index" (area under the midchord offset curve) and slope "changes."

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The defect detection scheme is the same in the Profile Program as in the Gage Program. When a profile value exceeds a selectable low threshold, such as -0.375 inch, a profile exception is indicated. Because profile anomalies are caused by low joints, and because lowjoint information is of short duration, there is no analysis done for grouping exceptions (i.e., all exceptions are point exceptions). Each time an exception is encountered, it is printed out in the appropriate rail column along with additional information about geographic location and severity (Figure 3-10).

Also included on the report page is a line entry for the segment summary whenever the appropriate amount of data has been analyzed. This summary presents data concerning the profile track quality of the segment and includes an analysis of the Index (area under the profile curve), Number of Slope Changes (abrupt change in profile value between adjacent samples), Mean, and Variance. These four quality measures are calculated for each rail.

The Profile Exception Location Summary Report (Figure 3-11) lists all locations sorted according to Index value. The Index value for each rail is summed for all the complete track segments within each location. Once the numbers have been accumulated, the total is divided by the number of track segments to obtain the average Index value per segment. The locations are then sorted by the severity of the average index number.

The purpose of this summary is to give an indication of the quality of track for track segments that average 5 to 10 miles. Planning can be developed for the maintenance of longer sections of track with this type of information.

Figure 3-8. Priority Profile Defect List (Chord Offset) Showing 100
Largest Profile Exceptions in Decreasing Order of Magnitude

Priority Profile Defect List (Track Segment Summary -
Index). A Similar List Can Be Prepared for Decreasing
Values of the "Slope" Index Number. Figure 3-9.

 $-$ - EXCEPTIONS - -SONTH - - SEGMENT SUMMARY (1320 FEET) - NORTH **NORTH** DIST FACT INDEX SLOPES MEAN **VAR** INDEX SLOPES MEAN VAR MP FEET MILE FEET SOUTH 77 $-.52*$ $\mathbf c$ 164 800 \overline{c} 3036 \overline{z} 58 3094 $\mathbf c$ **850** -0.38 ٠ 164 39 ${\tt GC}$ 3133 $-158+$ 164 900 \overline{c} ٠ \overline{c} $-.41$ $\mathbf c$ 164 1050 3304 $\mathbf c$ $\overline{2}$ $-.39$ 41 164 1100 3345 -42 24 \mathbf{c} 164 1150 \overline{c} 3370 1150 17 164 \overline{c} 3387 $-,46$ $\mathbf c$ 75 164 1200 \overline{z} 3461 $-.69**$ $\mathbf c$ 97 164 1300 \overline{c} 3558 $-.40$ $\mathbf c$ 164 1350 \overline{c} 3599 -0.42 41 $\mathbf c$ 164 1600 \overline{c} 3855 $-.50$ $\mathsf G$ -0.54 164 1700 \overline{c} 3954 1614 147 -0.34 $.0152$ 2233 253 $,0293$ 164 3000 \mathbf{r} 5259 $-.43$ $\mathbf c$ $-.024$ 164 3050 \overline{c} 5274 1218 80 -0.019 $.0097$ 1428 107 $.0127$ -0.49 164 3700 $\overline{\mathbf{3}}$ 644 951 $-.38$ 164 4000 $\overline{\mathbf{3}}$ 4050 $-.42$ 80 164 $\overline{\mathbf{3}}$ 1031 4100 $\mathbf{3}$ 1055 -0.39 24 164 1397 $\overline{\mathbf{3}}$ 107 7.016 $.0121$ 1468 120 $-.025$ $-01 - 2$ 164 4350 1313 3 1357 -0.38 164 4400 \sim 164 4550 $\overline{\mathbf{3}}$ 1500 -0.47 65 -0.41 164 4600 3 1565 164 4900 3 1877 -0.41 107 1538 $-.051$ $3, 2633$ 1410 $-.026$ $.0120$ 116 -0135 165 400 ø G 750 2988 $-0.54*$ 165 $\overline{\mathbf{3}}$ \bullet 165 900 $\overline{\mathbf{3}}$ 3162 -0.45 $-,59*$ $\mathbf c$ 165 1150 3397 \bullet $\mathbf{3}$ 127 $.0137$ 1469 165 1700 $\mathbf{3}$ 3952 1422 $-.023$ 128 $-.030$ $.0132$ 3 4259 $-.39$ 165 2000 36 165 2050 $\mathbf{3}$ 4296 -0.43 165 4682 $-.38$ $\mathbf c$ 2450 $\overline{\mathbf{3}}$ 165 2850 3 5096 $-0.55*$ G 77 165 2900 $3, 5173$ -0.38 1554 -0.633 122 -0.029 -0146 1672 134 $.0168$ 165 3000 3 5272 165 3050 \spadesuit 48 -0.47 \bullet 165 4100 $4 1063$ -0.38

LOCATION NUMBER : 06 TRACK NUMBER : 1

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PROCESSED 12/20/74 PROFILE FYALUATION ACOUISITION 41009-1W1 PAGE 2 $-.750***$ STANDARDS (INCHES) -.375 $-.625***$ $-0.875***$ MP161.8-MP170.5 $10C$ $x06$ $-0.500*$

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Figure 3-10. Detailed Profile Exception Report

* - EXCEPTION THRESHOLDS MAY VARY FROM LOCATION TO LOCATION

Figure 3-11. Profile Exception Location Summary Report Ranking
Locations by Magnitude of the Profile Index Value

The Profile Track Segment Summary Report (Figure 3-12) provides ^a more detailed Profile Exception Track Segment Summary. Track segments within each location are analyzed for severity of the index value. As with the Location Summary, the purpose of this section is to provide useful information for maintenance planning of larger sections of track.

Like the Gage Report, the Profile Report also displays relative track quality by means of data histograms. These graphical displays indicate quality by illustrating the distribution of profile measurements. The report provides one chord offset data histogram for each reference location derived from data combined from the two rails.

3.4 CURVATURE REPORT

The Curve Evaluation Program produces a report based on curve definition and FRA track safety requirements. It contains data similar to gage and profile reports for crisis maintenance, detailed exception analysis, and maintenance planning.

The Curve Evaluation Program generates four exception lists, with each one ordering the one hundred largest measurement exceptions in decreasing order of magnitude. The four measurements used are curvature, crosslevel, warp, and rock and roll (Figure 3-13).

The main curvature exception report gives a geographically ordered accounting of curves and exceptions as they are encountered during data collection. Curves are broken down into spirals and bodies (constant curvature). Exceptions are noted for crosslevel curvature mismatch, reverse crosslevel, curvature, crosslevel, warp, and rock and roll (Figure 3-14).

The detailed curvature exception list is divided into two sections: curve description and exception analysis (Figure 3-15). The detailed curve description completely defines each curve by location and magnitude, and gives point of spiral, spiral to curve, curve to spiral, and point of tangent, together with geographic location and overall length information. Also incorporated into the curve

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* = SEGMENT IS LESS THAN FULL TRACK SEGMENT

Figure 3-12. Profile Exception Track Segment Summary Report.
This Report Ranks Quarter-Mile Segments Within a
Location by Magnitude of the Profile Index Value

Figure 3-13. Curvature Priority Defect List. The Same Type
of List is Generated for Crosslevel, Warp, and
Rock and Roll Exceptions

Figure 3-14. Detailed Curve Evaluation Report

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Figure 3-15. Relationship of Detailed Curve Information
and the On-Line Strip Chart Display

display are statistics showing average superelevation, average curvature, and corresponding limiting and average speed information as defined by the following FRA Track Safety Standards equation which correlates curvature, elevation, and speed.

$$
V_{\text{max}} = \sqrt{\frac{E + 3}{0.0007d}}
$$
 where E = elevation
d = degree of curvature

$$
V_{\text{max}} = \text{maximum allowable}
$$

$$
V_{\text{max}} = \text{maximum allowable}
$$

The detailed curvature exception list defines six types of exceptions in terms of location, magnitude, and overall length (Figure 3-16). Crosslevel data are analyzed for possible crosslevel exceptions in tangent, spiral, or curve body areas. Warp is defined as the difference in measured crosslevel over a specified "warp" distance. Each warp condition is checked for a possible exception. Also defined is a reverse crosslevel exception condition (known as "undershoot" or "overshoot" at the start or end of a spiral). Another type of exception which is defined is rock and roll. This condition is determined by a difference in measured crosslevel over a specified distance and by a change in slope from one difference to the next.

Curvature exceptions are defined as those areas where curvature deviates from the average by more than ^agiven threshold. The exceptions are described in terms of magnitude and overall length.

The last type of exception compares crosslevel and curvature data to define potentially dangerous areas of superelevation-curvature mismatch. This anomaly, in which curvature occurs before superelevation, is defined by length-distance between the start of curvature and the start of superelevation, as well as by magnitudesuperelevation required for the measured amount of curvature.

R X-L = Reverse Crosslevel; $W = Warp$; X = Crosslevel; C = Curvature; R+R = Rock and Roll

Figure 3-16. Relationship of Detailed Exception Information
and the On-Line Strip Chart Display

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Summary information for curves is used in determining track quality and conditions of curves. Each entry into the Curvature Summary Report (Figure 3-17) represents each curve as defined in the detailed report. For each curve, all of the curve quantifying information is transferred from the detailed report along with information about the central angle traversed through the curve. An exception tally for each type of exception is also displayed for each curve.

3.5 INTEGRATED EXCEPTION REPORT

The Integrated Exception Program (INGRATE) incorporates data from different track geometry parameters into a single data reduction exception report. Whereas the three previously mentioned programs analyze individual parameters, INGRATE analyzes multiple track geometry parameters simultaneously and generates a multipurpose report. It contains data for detailed exception analysis and maintenance planning.

The INGRATE program uses the same analytic techniques for exception detection as are used in the Gage, Profile, and Curvature Programs. The parameters analyzed for anomalies are gage, profile, curvature, crosslevel, warp, and rock and roll. The Detailed Integrated Track Geometry Exception Report (Figure 3-18) displays exceptions for each parameter, with the capability of displaying one or more exceptions simultaneously if they occur at the same location on the track. Each exception is defined in terms of geographic location, magnitude, and overall length. The primary importance of this report is that the detailed information contained in the three preceding reports are combined to allow the user to simultaneously observe anomalies for multiple track geometry parameters.

The One-Mile Exception Summary Report displays the number of exception samples for each parameter processed. Two columns are provided for each parameter. The first contains the number of exception samples per mile and the second presents the count of exception samples using a different exception threshold. Thus, in the summary

CURVE SUMMARY LOCATION : 16

FRA TRACK CLASS : 2

TRACK NUMBER : 1

FOR CUPVATURE--POSITIVE VALUE=CENTER OF CURVATURE TO WEST NEGATIVE VALUE=CENTER OF CUPVATURE TO EAST FOR CROSSLEVEL--POSITIVE VALUE= EAST PATI HIGH NEGATIVE VALUE= WEST RAIL HIGH

Figure 3-17. Curve Summary Report. Report Displays
a Summary of Every Curve Encountered During a Test Run

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Figure 3-18. Detailed Integrated Track Geometry Exception Report. This Report Displays Exceptions for Each of Six Different Measurement Parameters.

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displayed in Figure 3-19, the first column under Gage shows the number of samples which exceed a 57.00-inch threshold, while the second column shows the number of samples that exceed a less stringent threshold. This allows two different analyses of the same section of track.

3.6 INTEGRATED STANDARDS REPORT

The Integrated Standards Program was developed to analyze track geometry data for exceptions to the track safety standards as defined by the FRA Office of Safety. Parameters are evaluated according to track safety standards in a detailed exception analysis and maintenance planning format. Besides the computer output, the program generates a file of information which can be subsequently used for a graphic display of the data.

This program uses the same set of parameters as the Integrated Exception Program. However, curvature and rock and roll are not analyzed for exceptions (Figure 3-20). Curvature data are used for determining geographic location of track curves. Once the track type (tangent, spiral, curve) has been established, this information is used to determine which set of thresholds is required for the analysis of the data. Profile data are taken from the signal of the inertial profilometer and converted to 62-foot chord information. Warp is evaluated for anomalies for distances between 2-1/2 feet (single sample length) and 62 feet.

As with the other reports, each parameter exception is defined by geographic location, magnitude, and in most cases overall length. The length displayed for a warp exception represents the distance which has produced the maximum anomaly found in a 62-foot window area.

As with the Integrated Exception Program, the Integrated Standards Program generates a one-mile exception summary which displays the number of exception samples for each parameter processed (Figure 3-21). For each parameter, two columns provide the capability to

Figure 3-19. One-Mile Exception Summary Report. Report Displays Per Mile the
Number of Exceptions/Severe Exceptions Found for Six Measured
Parameters

 $A \cap \{x \in Y + T \cap U \mid X \leq 1, 2, 3, 5\}$

Figure 3-20. Integrated Track Geometry Safety Standards Report. Report Displays all
Exceptions to FRA Track Safety Standards for Five Measured Parameters

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DATA FROM MP640 TO MP607
TRACK NUMBER : 1

Figure 3-21. One-Mile Exception Summary Report. Report Lists Per Mile the Number of Operating Class and Class 1 Exceptions for Each of Five Measured Parameters.

look at the number of exception samples per mile for two different sets of thresholds. As an example, it is possible to observe the number of defects according to appropriate track class while at the same time determining how many miles of track need immediate maintenance because of the presence of exceptions exceeding Class 1 standards.

The One-Mile Class Summary gives information on compliance with safety standards once deviations have been located. The highest allowable operating class is displayed per mile for each parameter processed. Also printed for each mile is the operating class as set by the railroad and the overall limiting class as determined by parameter comparison with safety standards. Other speed information displays any restrictions found from the comparison of elevation and curvature on every curve within each mile. Figure 3-22 gives an example.

The Curve Summary (Figure 3-23), which shows the points of change from tangent to spiral and spiral to curve for each mile, is included as an aid in determining where curves have been located for changes in class standards.

As mentioned above, one added feature of this program is that it has the capability for generating a graphic display of all information provided in the summaries. In Figure 3-24, the top four ^plots show the number of posted class and Class 1 exceptions for each of four parameters for each mile tested. The fifth plot dis^plays the railroad operating speed along with any limiting curve speeds as defined by the FRA Track Safety Standards. The last plot presents the operating class and the highest allowable class as determined by the severity of deviations for the parameters analyzed.

3.7 DATA COMPARISON REPORTS

Data comparison reports are used to provide relative comparisons between different sets of track geometry data for higher level maintenance planning. Two programs (COMPARE and GEOPLOT) have been developed for this purpose.

ONE MILE CLASS SUMMARY REPORT
DATA FROM MP640 TO MP607

TRACK NUMBER : 1

CLASS 0 CLASS 1 CLASS 2 CLASS 3 CLASS 4 CLASS₅ CLA55 6

 $\mathbf{1}$ $\overline{\mathbf{3}}$ 14 9 \cdot \mathbf{I} $\mathbf 0$

TOTAL POSTED MILES PER CLASS

TOTAL MILFS COVERED IN REPORT= 37

Figure 3-22. One-Mile Class Summary Report. Report Displays Per Mile the Maximum
Allowable Class for Four Parameters Plus the Overall Allowable Class

DATA FROM MP640 TO MP607

Figure 3-23. Curve Summary Report. Report Gives Summarized Curve Information for Each Curve Encountered

Figure 3-24. Integrated Standards Graphic Display. Display Shows Operating Class and Class 1 Exceptions for Four Parameters. These are Combined to Show Posted and Safe Operating Class for Each Mile of Tested Track.

 $\ddot{\mathcal{S}}$ Vl Vl The COMPARE Report provides a statistical comparison between two sets of track geometry data. Various statistical measures of each data set are included in addition to a histogram overlay plot and a Power Spectral Density plot of each data set. This data processing program was developed to compare track geometry statistics and present conclusions in both graphical and tabular format. This analysis can be made with either one or two sets of data. Two sets of data are used to display a historical trend in the condition of the track (Figure 3-25).

The histogram shows not only the percentage of measurements which have exceeded the exception thresholds, but also the percentage of measurements which are on the verge of becoming exceptions. It also shows how closely the measurements are clustered about the design value.

Power Spectral Density (PSD) plots (Figure 3-26) are commonly used in the characterization of electrical signals varying randomly with time, as the area contained under the PSD curve between any two frequencies can be related to the total electrical power delivered by the random signal within that bandwidth. Similarly, the area between any two frequencies under the track geometry PSD curve is related to the dynamic energy delivered to a traveling vehicle by the track. Track geometry PSD's are useful both as track quality indicators and as inputs to dynamic models for vehicle response analysis because of the amplitude versus wavelength information content.

Single data set histograms and PSD's can also be used to provide ^a graphical comparison of different types of track. The gage histograms and profile PSD's shown in Figure 3-27 are examples of the type of information that can be presented.

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Figure 3-25. Comparison Report - Profile Histogram Example

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Figure 3-36. Comparison Report - Profile PSD Example $3 - 36$

Figure 3-27. Examples of Single Data Set Histogram and PSD Comparisons

GEOPLOT is a data processing program used to compare data from historical test runs. The object of the report is to provide a visual display that will assist maintenance-of-way personnel in determining whether a specific section of track has degraded as a result of little or no maintenance or has improved as the result of maintenance actions taken between runs. Some railroads are using this repor^t as ^atool for evaluating the status of track maintenance and the effectiveness of maintenance equipment, and for determining the extent of track degradation. Visual identification of changes in the condition of track over several years is a valuable feature of the report.

The GEOPLOT Report produces a plot which displays up to four computed track quality measures from up to three separate test runs over the same trackage. The program overlays the data from the multiple surveys and displays the computer measurement on a mile-by-mile basis (Figure 3-28).

TRACK PARAMETER QUALITY INDEX CHART

PROCESS DATE - 12/19/74
X=2 OR HORE POINTS THE SAME
N=8AD OR NO DATA

MP603 IO MP741
3 = 1973 DATA
4 = 1974 DATA

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GEOPLOT Report. This Report Shows Quality Index
Comparisons of 50 Miles of Data for Two Different
Years Figure 3-28.

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PAGE 1

4.0 **SYSTEr1 VALIDATION AND QUALITY INDEX DEVELOPMENT**

4.1 INTRODUCTION

Studies were performed under the Rail Research Program to promote better utilization of the data gathered by the FRA measurement cars and to investigate new methods and techniques for solving rail research problems. One of the most significant studies was an extensive track geometry system validation effort which verified the accuracy of track geometry measurements. Other studies included efforts to examine the relationship of track geometry to ride quality; generate ride quality indices from track geometry data; and to study the repeatability of index generation.

4.2 FRA TRACK GEOMETRY MEASUREMENT SYSTEM VALIDATION

An extensive testing program was conducted to validate the track geometry measurement system installed on the FRA measurement cars to establish the accuracy and repeatability of measurements. Comparisons were made between manual and high-speed electronic measurements of rail gage, crosslevel, profile, and alignment.

The lack of a set of absolute reference standards made it necessary to include not only field tests of the systems, but also controlled laboratory tests of various components. The field tests permitted comparison of system measurements with manual measurements, as well as the evaluation of system repeatability under various operating speeds, car directions, and types of track.

Analysis of field test results provided information on system performance, while the laboratory tests were designed to evaluate the operational characteristics of various components in an absolute sense.

System repeatability studies and comparisons with manual measurements were made by calculating point-to-point measurement differences and performing statistical analysis on the set of differences. ^A summary of the validation results is presented in Table 4-1.

It should be noted that if the variances of the manual measurement and the electronic repeatability are combined, they nearly equa^l the variance associated with manual versus electronic measurement differences. This agreement supports the conclusion that system measurement is generally more accurate than manual measurement.

The results provide estimates on the average performance of the measurement subsystems over ^along section of track. Special local conditions may cause increased errors in the measurements at isolated points or segments of track. The effect of such conditions, including worn rails, joint bars, etc., is discussed in detail in the report entitled "Track Geometry System Validation Report," DOT-FR-73-08.

4.3 RELATIONSHIP OF TRACK GEOMETRY AND TRACK QUALITY

Track quality measures show the measurement of track degradation and the determination of maintenance needs. A number of measures were developed and investigated for reliability, repeatability, and significance, and the Bessemer and Lake Erie (B&LE) and the Denver and Rio Grande Western (D&RGW) railroads have studied several measures for use in maintenance planning. These track quality measures are derived from gage, profile, crosslevel, and curvature parameter measurements.

^Alist of measurements and definitions is included in Appendix A. Those currently under investigation are: Standard Deviation, Slopes, and Index. Standard Deviation is calculated as the square root of the statistical variance, Slopes as the number of significant changes between two adjacent data samples, and Index as the area under the data curve.

TABLE 4-1a SUMMARY OF SYSTEM PERFORMANCE DATA

TABLE 4-1b SUMMARY OF VALIDATION RESULTS

		Accuracy	
	Maximum Range of Measurement	Electronic System Repeatability	Variations from Manual Measurements
GAGE	$55.5" - 61.5"$	±0.025"	±0.05"
PROFILE*	$+2$ "	$+0.025$ "	$+0.04$ "
ALIGNMENT*	$+2"$	$+0.030$ "	$+0.05$ "
CROSSLEVEL	$+6.5"$	$+0.030$ "	$+0.08$ "

*Mid-chord offset using a 14.5-foot chord.

A number of experiments have been performed to investigate track quality measures. One of the investigations involved a test run on the B&LE Railroad to relate track quality indices to track ride quality as rated by two experienced railroad supervisors.

The supervisors were asked to rate the quality of ride of each section of track measured, although neither the rating scale nor the exact meaning of "ride quality" was explicitly defined. Each section was rated by the supervisors as they rode the track geometry car over different sections of track during three days of testing, by using a scale of 1 (for excellent ride quality) to ¹⁰(for poor ride quality). Since railroad reported the numerical average of the pair of ratings for each section, no analysis was made on the degree to which the railroad supervisors agreed on their ratings. All of the track was continuously welded rail maintained for 45-mph freight operation.

^Acomputer was used after the test run to calculate a number of different track quality measures for each section of track based on recorded profile and gage measurements. These measures were then compared with the ride quality ratings made by the supervisors during the test run.

Each of the measures calculated from profile measurements ranked the track in approximately the same order as that selected by the supervisors. The track geometry measure that best correlated with ride quality rankings was calculated by counting abrupt profile changes (changes greater than 0.1 inch between adjacent data samples). This measure, "Slopes Per Mile," exhibited an absolute average error of less than 10 percent and a maximum single error of less than 20 percent of the ride quality ratings assigned by the railroad supervisors. (See Table 4-2.)

None of the measures calculated from gage measurements correlated well with the ride quality ratings. Gage apparently had little effect on ride quality, at least within the range of gage conditions found on the B&LE main line.

TABLE 4-2

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DISTRIBUTION OF ABSOLUTE ERROR TERMS USING THE SIMPLIFIED SLOPES PER MILE RELATIONSHIP

Average Difference 0.94

Further information can be found in the report titled "Correlation Between Track Geometry Indices and Perceived Ride Quality," Report No. DOT-FR-73-15.

A second experiment was conducted on the D&RGW railroad. Track geometry data were processed for many sections of trackage where known maintenance had been performed between successive track geometry surveys. Selected track geometry measures were then calculated for each mile of track in these maintenance control zones. Graphic presentations of the measures were used to enable the qualitative comparisons of the consistency and responsiveness of the measures to several maintenance levels. Figure 4-1 displays an example of the graphic presentation. In the figure six sections of track show the responsiveness of the Profile Slopes measure to the maintenance action of laying new rail. Various gage and profile measures were tested using three types of maintenance: new rail, spot raise, and skin lift. The results showed that Standard Deviation, Slope, and Index were the measures which were most consistent and responsive to different types of maintenance.

The study of the correlation of track geometry indices to human judgment and known track maintenance improvements has shown that stable correlations do exist. Additional investigations will provide the basis for the evaluation of the usefulness of track geometry indices for track maintenance planning. Although it is not suggested that track geometry measurement cars replace the judgment of experienced supervisors, it does appear that such cars can be used as a tool by these supervisors to provide reliable and accurate measures of ride quality useful in the process of allocating major track maintenance.

4.4 TRACK GEOMETRY MEASUREMENT AND MEASURES REPEATABILITY

There are four major causes for differences in the track geometry measurements at any point along the track at two different times.

Figure 4-1. Graphic Display of Quality Measure Difference for Certain Types of Maintenance

- Sample error--the track geometry is sampled at increments of 2.41 feet during each test, so two measurements assumed to be made at the same point may actually be made at points up to 1.205 feet apart.
- System noise--including random vibration of the sensors, round-off errors in digitizing, system noise and nonlinearity, etc.
- Calibration errors.
- Changes in the track geometry.

The ability of the track geometry system to measure track degradation or change depends on the repeatability of the measurements and the indices calculated from the measurements. To check the repeatability of the system, four test runs were made over a $6-1/2$ mile section of B&LE mainline over a period of approximately $1-1/2$ hours. Since it can be assumed that the track geometry did not change during this test, the differences from one run to the next are an indication of the degree of repeatability of the measuring system. This test essentially eliminated the influences of calibration errors and changes in track geometry upon measurement repeatability.

^Apreliminary analysis has resulted in an estimate of the magnitude of the other sources of error. These imply a confidence interval for individual point measurements which is as follows:

^Asimilar repeatability analysis will be performed for the profilometer. This will allow comparison of the profilometer to the beam profile system, and will allow evaluation of the effects of different chord lengths used to calculate mid-chord offset.

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5.0 SYSTEM DESCRIPTION

5.1 INTRODUCTION

This chapter provides a nontechnical description of the track geometry measurement system installed onboard the FRA measurement
cars. Data processing requises and is an Data processing provides real-time computation and reporting of track geometry characteristics, and digital recording of both raw and computed data.

5.2 BASIC SYSTEM CONCEPT

The major subsystems and signal flow of the FRA Track Geometry Measurement System are shown in Figure 5-l. As shown, each subsystem is configured to measure, record, and display a particular parameter of track geometry. The parameters include:

- Profile
- Alignment
- Gage

• Cross level

- - - - - - - - - - - - -

- Curvature
- Location

Each subsystem utilizes one or more sensors, signal processors, ^a digital computer, recording equipment for digital magnetic tape, and display equipment which provide on-line printout and analog
reports. The components associated with each and in the same The components associated with each subsystem are discussed in the following paragraphs.

5.3 PROFILE AND ALIGNMENT MEASUREMENT SUBSYSTEM (Figure 5-2)

Mid-Chord Offset (MCO) profile and alignment sensors are located in pairs at six locations on the two 14.5-foot truck-mounted beams. The sensors are arranged in pairs, and are located at the ends and center of each beam. The beams are situated so that the sensors

5-l

are in proximity to the top of each rail. Each sensor is connected in a capacitive feedback loop to an instrumentation amplifier insid'e the car. Each sensor amplifier produces an output signal that is proportional to the distance between the rail and sensors, which is dependent upon:

- Vertical distance from the sensor to the rail
- Lateral displacement of the sensor with respect to the rail

MCO profile computations are based on the vertical distance from a pair of sensors (i.e., LOR, LIR) to the surface of the rail, while alignment computations are based on differences in lateral displacement of the two sensors which constitute a pair.

^Atypical sensor pair is shown in Figure 5-3. The arrangement of sensor pairs on the two beams is shown in Figure 5-4.

Figure 5-l. Track Geometry Measurement System Block Diagram

Figure 5-2. Profile and Alignment Subsystem Block Diagram

Typical Capacitive Sensor Pair Figure 5-3.

Figure 5-4. Location of Capacitive Sensors on Truck-Mounted Beam

Rail curvature in the horizontal and vertical planes can be determined using a mid-ordinate to chord measurement technique. This technique utilizes the displacement of the mid-point of an arc from the mid-point of the chord drawn between the end points of the arc as a measure of the curvature. As shown in Figure 5-5, if P_1 and P_3 are the end points of a chord and lie on the arc, then D is the displacement of the mid-point of the arc from the (P_2) of a chord, or D is the mid-ordinate to chord measure of the curvature of the arc. If P_1 and P_2 do not lie on the arc, then the mid-ordinate to chord measure (D) becomes:

$$
D = \frac{(a-b) + (c-b)}{2}
$$

$$
= \frac{a-2b+c}{2}
$$

Figure 5-5. Mid-Ordinate To Chord Measurements

Since the capacitive sensors on the FRA measurement car are noncontacting, this equation is used to compute the mid-ordinate to chord measure of the vertical (profile) and lateral (alignment) rail curvature.

The signals developed by a pair of sensors are then combined to form one signal for front, rear, and mid-chord points. The vertical distance from the rail top to each capacitive sensor pair is a nonlinear function of the sensor output voltages. A bilinear function is used to closely approximate the calibration curves determined in laboratory tests.

The three vertical distances measured by the three sensor pairs on each beam are then used to compute the mid-chord offset (MCO) for profile. Details of profile data processing are covered in "Profile Program Manual," DOT-FR-73-05.

While the MCO measurement technique is an effective method of measuring rail profile, certain restrictions are imposed due to the characteristic frequency response of the system. The frequency response of the FRA MCO system is shown in Figure S-6. It can be seen that the response is null at wavelengths of 1/2, 1/4, 1/6, 1/8 ... of the chord length, and double at wavelengths of 1/3, 1/5, 1/7, 1/9 ... of the chord length. However, the profilometer exhibits a flat frequency response throughout the wavelength range of interest. For that reason, an inertial profilometer has recently been added to increase the FRA Test Car profile measurement capability. The inertial profile sensors (profilometers) are mounted on the car truck on each side of the car. The profilometers establish an inertial reference and develop signals which are proportional to deviations from that reference.

The profilometer is basically composed of a mass which is attached to a wheel of the test car through a spring and damper assembly (Figure S-7). The mass motion is restricted to the vertical direction by low-friction guides. An accelerometer is attached to the mass, and a displacement transducer is connected between the mass and vehicle wheel (axle).

Figure S-6. Frequency Responses of MCO and Profilometer Systems

Figure 5-7. Principles of Profilometer Operation

As the vehicle moves along a track, the mass acts as an inertial reference in the vertical plane. Vertical displacements of the rail act as inputs to the profilometer, and are measured directly by the displacement transducer. Low-frequency inputs, which fall below the natural frequency of the profilometer mechanism, are measured by double-integration of the output of the accelerometer, and then adding the integrated signal to that developed by the displacement transducer.

Due to mounting space limitations on the FRA measurement cars, the profilometer transducers are not mounted directly over the rails. Thus, the point of measurement (rail center) extends beyond the actual location of the profilometer transducer, as shown in Figure 5-8. This condition is termed "overhang," and is represented by distances B and C in the illustration. The signal produced by each transducer does not represent the true profile of the rail but, rather, is a scaled linear combination of

profile of both rails. As shown in Figure 5-9, actual profile $(Z_R \text{ and } Z_L)$ is always greater than the measured signals $(Z_{RT} \text{ and } Z_L)$ $Z_{L,T}$). This condition is corrected by compensation circuits consisting of operational amplifiers with gain characteristics set to provide the required output. A block diagram of the profilometer instrumentation is shown in Figure 5-10.

Location of Profilometer Sensors Figure $5-8$. Relative to Rail Center

Graphical Representation of Overhang Figure 5-9. Showing Left Rail High

Figure 5-10. Profilometer System Block Diagram

5.4 GAGE MEASUREMENT SUBSYSTEM (Figure 5-11)

The gage sensors, which are also capacitive proximity devices, are mounted in the shadow of the wheel flanges on the 14.5-foot truckmounted beams. The sensors face the inside of the rails and measure at a point approximately 5/8 inch below the top of the rail. The signals produced are proportional to the distance between the face of the sensors and the inside of the railhead. The signals are then processed by signal conditioning amplifiers, and are routed through the analog terminal unit to the analog multiplexer of the digital computer. The data are sampled and stored in the same manner as the profile and alignment MCO signals previously discussed.

The distance between the faces of the left and right gage sensors is referred to as faceplate distance (Figure 5-12). Therefore, addition of the implied distance from the right sensor to the right rail (RG), the implied distance from the left sensor to the left rail (LG), and the faceplate distance (FPD) provides the measurement of gage.

The two gage gaps are combined with the faceplate distance using the following equation:

$Gage = LG + RG + FPD$

The capacitance between the rail and the sensor is a nonlinear Therefore, a linearization function of the distance between them. process consisting of a five-degree power series is used to represent the nonlinear relationship between the output voltage of each sensor and the distance (gap) between the sensor and rail. Details of gage processing are covered in "Gage Program Manual," DOT-FR-72-03.

Figure 5-11. Gage Measurement Subsystem Block Diagram

5.5 CROSSLEVEL MEASUREMENT SUBSYSTEM (Figure 5-13)

The Crosslevel Measurement Subsystem measures the difference in elevation in inches, between the right and left rails. The subsystem consists of a self-erecting gyroscope, two displacement transducers (linear potentiometers--one mounted on each side of the car), and analog circuitry to process the signals.

The vertical gyroscope is the principal reference for the Crosslevel Measurement Subsystem. It is floor-mounted in Car T-3, above the "A" truck, and provides an output voltage which is proportional to the roll angle of the car body from absolute vertical. Since there is a fixed relationship between degree of inclination and crosslevel, only simple conversion and correction are required to obtain crosslevel measurement directly.

Since'the body of the car is free to roll through bolster action with respect to the truck, an angular correction is required to determine the roll angle of the truck. The two displacement transducers are used for this correction, and are positioned to measure the difference in height between each side of the car body and the truck. Figure 5-14 shows the relationship of the gyro and transducer locations.

The crosslevel computer is an analog circuit which generates an output signal proportional to crosslevel. A positive output voltage denotes left rail high; a negative voltage denotes right rail high. The output of the crosslevel computer is input to the analog multiplexer of the digital computer. Digitized crosslevel data are stored for recording on magnetic tape and are directly displayed on a strip chart recorder. Figure 5-15 displays the crosslevel computer technique.

Figure 5-13. Crosslevel Measurement Subsystem Block Diagram

Figure 5-14. Relationship of Gyroscope, Displacement Transducers and Car Body

5-12

 $\mathcal{L}_{\mathrm{max}}$, and $\mathcal{L}_{\mathrm{max}}$

 \mathfrak{g}^{\pm}

 $Crosslevel = (Z2+Z4) - (Z1+Z3) + KSIM\Theta$

Figure 5-15. Crosslevel Computation Technique

5.6 TRACK CURVATURE MEASUREMENT SUBSYSTEM (Figure 5-16)

The Track Curvature Subsystem measures the curvature of track in degrees per 100 feet. The system employs an inertial rate gyroscope to measure the yaw rate of the car, a speed signal from the Speed and Distance Processor, and two velocity transducers to measure relative yaw motions between the car and the trucks.

The curvature signal is digitized and recorded on magnetic tape, and is displayed in analog form on a strip chart recorder to provide a visual display and permanent record of the measurement.

5.7 AUTOMATIC LOCATION DETECTOR SUBSYSTEM (Figure 5-17)

The Automatic Location Detector (ALD) subsystem is used to detect physical features which are unique to a particular section of track roadbed. The detected features are used to correlate track geometry data with specific physical location. The ALD sensor

Figure 5-16. Curvature Measurement Subsystem Block Diagram

is located on ^aframe approximately in the center of the "A" truck on Car T-3 (Figure 5-18), and detects the proximity of physical objects in the centerline of the track roadbed.

The ALD signal is sensed and recorded simultaneously with the track geometry data. The signal is then digitized, displayed on ^astrip chart recorder, and stored on magnetic tape (Figure 5-19).

There are several types of ALD indications displayed on the strip chart recorder. These are as follows:

- MILEPOST: Indication that appears as ^asmall negative pedestal when initiated by the control operator.
- LOCATION: Indication that appears as a positive signal excursion superimposed on a computer-generated pedestal. The positive pedestal is initiated by the control operator. This indication is used to locate a specific geographic position on the track.

ROAD CROSSING and TURNOUT: Indications that appear as positive signal excursions on the ALD trace. The two are differentiated by the shape of the waveform.

In planning a test run, objects such as road crossings, turnouts, etc., are assigned three-digit codes. As these objects are approached during a test run, an identifying code is entered in the remote control unit by sequentially depressing the three numerical digits of the code. As the object of interest passes under the leading vehicle, the code is keyed into the computer. When detection of the object is made by the ALD sensor, the code is recorded on the digital tape.

Figure 5-17. ALD Subsystem Block Diagram

Figure 5-18. Location of ALD Sensor on Measurement Car

Figure 5-19. Typical ALD Strip Chart Trace

5.8 SPEED AND DISTANCE MEASUREMENT SYSTEM

An optical tachometer, chain-driven by the car axle, produces one thousand pulses during each revolution of the train wheel. The pulses produced by the tachometer are counted over a fixed time interval by the speed and distance processor to indicate instantaneous vehicle speed, and are accumulated by the same unit to indicate total distance of vehicle travel. The speed and distance processor provides the primary timing signals which control data acquisition, computer sampling, and data recording. Speed and timing signals are also distributed throughout the entire track geometry measurement system where measurement functions are related to vehicle speed. A functional block diagram of the spee^d and distance subsystem is shown in Figure 5-20.

5.9 ANALOG DISPLAY SUBSYSTEM

The Analog Display Subsystem consists of two strip chart recorders, associated preamplifiers, and data channel selection equipment.

The analog signal from each measured track geometry parameter can be displayed on a strip chart recorder to provide a real-time visual display and a permanent record of data in an analog form. The eight-channel recorder resides with the computer and associated data acquisition units to allow for monitoring incoming geometry data (Figure 5-21). The movable six-channel recorder is placed in the rear vestibule of the final car in the consist to allow visual correlation between strip chart data and significant track features by railroad personnel during a test run.

Figure 5-21. Track Geometry Computer System

5.10 DIGITAL RECORDING SUBSYSTEM

The Digital Recording Subsystem records all pertinent track geometry data in digital form on magnetic tape to provide a permanent record, and provides data for off-line processing on a large computer system. Such off-line processing results in reports and data tabulations for further analysis of specific aspects of track geometry.

As track geometry data are measured, the primary measurement signals produced are analog voltages. The analog voltages are scanned, digitized, and stored temporarily in the digital computer. When ⁸⁰ scans of data have been digitized and stored, the data are then transferred to magnetic tape in ^acomplete block. One scan of data is initiated by each block distance pulse, which is supplied by the speed and distance processor each 2.41 feet of vehicle travel.

A maximum of 32 channels are used as analog inputs to the data collection system. The channels are digitized before being input to the computer.

Fifteen of the analog inputs are committed to proximity sensors of the measurement system; twelve channels are assigned to the profile and alignment sensors; two channels are assigned to the Gage sensors; and one channel is assigned to the ALD sensor. Five additional signals, including crosslevel, curvature, speed, and left and right inertial profile, are also input to the computer. The remaining channels are available for special test data collection needs.

5.11 ON-LINE REPORTS

The Track Geometry Measurement System presently generates three separate types of reports for operating personnel and has the capability of supplying additional types of reports as required. The following reports are presently provided.

The Exception Listing Report (Figure 5-22) contains information on track geometry parameters which exceed a predetermined range of acceptable values. The first, last, and maximum variations in excess of acceptable limits are printed and marked with respect to the last milepost and distance in feet from that milepost. The report also includes a summary of the mean and standard deviation of the values since the last summary, and an index value associated with each value. The index value is proportional to the product of the variation from the mean multiplied by the length of variation.

Figure 5-22. Typical Exception Listing Report

APPENDIX A TRACK QUALITY MEASURES

- x_i = Gage in inches at point i on the track
- n Number of gage measurements made over section of track

GAGE:

- 1. Mean:
	- Mean = $1 \frac{1}{2}$ $\overline{n}_{i=1}^{\lambda}$ x_i $\overline{\mathrm{x}}$
- 2. Standard deviation: Standard deviation = $[\text{variance}]^{1/2}$

where variance = $\frac{1}{n-1}$ [$\sum_{i=1}^{n} (x_i - \overline{x})^2$] = $\frac{1}{n-1}$ n [I $\sum_{i=1}^{8}$ $(x_i)^2 - n\overline{x}^2$

- 3. Slopes: n Slopes = Σ $F(x_i)$ $i=1$ Where $F(x_i) = 0$ if $|x_i - x_{i-1}| < 0.1$ = 1 if $|x_{i} - x_{i-1}| \ge 0.1$
- 4. Index $(-)$: Index (-) = $\sum_{i=1}^{n}$ [F(x_i) · SI] Where $F(x_i) = x_i - 56.5$ inches SI = Sample Interval = 2.41 feet

5. Index (0):
\nIndex (0) =
$$
\sum_{i=1}^{n} [F(x_i) \cdot SI]
$$

\nWhere $F(x_i) = 0$ if $x_i \le 56.5$ inches
\n= $x_i \cdot 56.5$ if $x_i > 56.5$ inches
\nSI = Sample Interval = 2.41 feet

^Ahistogram of gage data is used for calculating the 5% Lt. and 5% Rt. measures.

6. 5% Lt.:

5% of the gage data lies below this gage value.

7. 5% Rt.:

5% of the gage data lies above this gage value.

- x_i Profile in inches offset from 14.5-foot chord at point ¹ on the track
- n ⁼Number of profile measurements made over section of track

PROFILE:

1. Mean:
\nMean =
$$
\frac{1}{n}
$$
 $\sum_{i=1}^{n} x_i = \overline{x}$

A- 2

The Informative Message Report lists Forward Observer commands, information related to tape starts, parity errors, and location detections. The report also lists abnormal sensor voltages and extreme track geometry parameter exceptions. Each message entry on the list is tagged with the last milepost number and the distance in feet from the milepost.

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The Urgent Message Report is designed to inform operating personnel of unusual, abnormal, or extreme system operating conditions. Such conditions include abnormal sensor voltages, abnormal voltage changes, and extreme geometry exceptions.

 $\label{eq:2.1} \frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^{2} \left(\frac{1}{\sqrt{2}}\right)^{2} \left(\$ $\mathcal{L}^{\mathcal{L}}_{\mathcal{L}}$ and $\mathcal{L}^{\mathcal{L}}_{\mathcal{L}}$ and $\mathcal{L}^{\mathcal{L}}_{\mathcal{L}}$ $\label{eq:2.1} \mathcal{L}(\mathcal{L}(\mathcal{L})) = \math$ $\frac{1}{4} \left(\frac{1}{\sqrt{2}} \right)^{2} \left(\$

2. Standard deviation:

Standard deviation = $[variance]$ ^{1/2}

Where:

variance =
$$
\frac{1}{n-1}
$$
 $\left[\sum_{i=1}^{n} (x_i - \overline{x})^2\right] = \frac{1}{n-1} \left[\sum_{i=1}^{n} (x_i)^2 - n\overline{x}^2\right]$

- -----------------

3. Slopes:

Slopes = $\sum_{i=1}^{n}$ F(x_i) i =1 $^{\mathrm{-1}}$ Where: $F(x_i) = 0$ if $|x_i - x_{i-1}| < 0.1$

= 1 if
$$
|x_i - x_{i-1}| \ge 0.1
$$

4. Index:

$$
\begin{array}{rcl}\n\text{Index} & = & \sum\limits_{i=1}^{n} \left[F(x_i) \cdot \text{SI} \right]\n\end{array}
$$

Where $F(x_i) = |x_i|$

SI = Sample Interval = 2.41 feet

Histogram of profile data is used for calculating the 5% Lt. and 5% Rt. measures.

5. 5%Lt.:

5% of the profile data lies below this profile value.

6. 5% Rt. :

5% of the profile data lies above this profile value.

- 7. SIGMA= Measured value of profile corresponding to 0.26% on the cumulative histogram.
- 8. DM/0.375 = Average number per mile of profile measurements less than -0.375 inch.

APPENDIX B

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